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## Circular Pipe Head Loss Equation (Head Loss Expressed

 in Feet)$$
h_{f}=\frac{4.73 L}{C^{1.852} D^{4.87}} Q^{1.852}, \text { where }
$$

$h_{f}=$ head loss (ft)
$L=$ pipe length (ft)
$D=$ pipe diameter (ft)
$Q=$ flow (cfs)
$C=$ Hazen-Williams coefficient
Circular Pipe Head Loss Equation (Head Loss Expressed
as Pressure) as Pressure)
U.S. Customary Units

$$
P=\frac{4.52 Q^{1.85}}{C^{1.85} D^{4.87}}, \text { where }
$$

$P=$ pressure loss (psi per foot of pipe)
$Q=$ flow (gpm)
$D=$ pipe diameter (inches)
$C=$ Hazen-Williams coefficient

## SI Units

$$
P=\frac{6.05 Q^{1.85}}{C^{1.85} D^{4.87}} \times 10^{5}, \text { where }
$$

$P=$ pressure loss (bars per meter of pipe)
$Q=$ flow (liters/minute)
$D=$ pipe diameter (mm)

| Values of Hazen-Williams Coefficient $\boldsymbol{C}$ |  |
| :--- | :---: |
| Pipe Material | C |
| Ductile iron | 140 |
| Concrete (regardless of age) | 130 |
| Cast iron: |  |
| $\quad$ New | 130 |
| 5 yr old | 120 |
| 20 yr old | 100 |
| Welded steel, new | 120 |
| Wood stave (regardless of age) | 120 |
| Vitrified clay | 110 |
| Riveted steel, new | 110 |
| Brick sewers | 100 |
| Asbestos-cement | 140 |
| Plastic | 150 |

## TRANSPORTATION

U.S. Customary Units
$a=$ deceleration rate $\left(\mathrm{ft} / \mathrm{sec}^{2}\right)$
$A \quad=$ absolute value of algebraic difference in grades (\%)
$e \quad=$ superelevation (\%)
$f \quad=$ side friction factor
$\pm G=$ percent grade divided by 100 (uphill grade " + ")
$h_{1} \quad=$ height of driver's eyes above the roadway surface (ft)
$h_{2}=$ height of object above the roadway surface (ft)
$L=$ length of curve ( ft )
$L_{\mathrm{s}} \quad=$ spiral transition length ( ft )
$R=$ radius of curve ( ft )
$S S D=$ stopping sight distance (ft)
$t \quad=$ driver reaction time (sec)
$V=$ design speed (mph)
$v \quad=$ vehicle approach speed (fps)
$W$ = width of intersection, curb-to-curb ( ft )
$l=$ length of vehicle ( ft )
$y=$ length of yellow interval to nearest 0.1 sec ( sec )
$r \quad=$ length of red clearance interval to nearest $0.1 \mathrm{sec}(\mathrm{sec})$

## Vehicle Signal Change Interval

$$
\begin{aligned}
& y=t+\frac{v}{2 a \pm 64.4 G} \\
& r=\frac{W+l}{v}
\end{aligned}
$$

## Stopping Sight Distance

$S S D=1.47 V t+\frac{V^{2}}{30\left(\left(\frac{a}{32.2}\right) \pm G\right)}$

